

POTENTIAL IMPACTS OF WEATHER AND TRAFFIC CONDITIONS ON ROAD SURFACE PERFORMANCE IN TERMS OF FOREST OPERATIONS CONTINUITY

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Abstract. The aim of this study was to evaluate the changes in forest road pavement bearing capacity (PBC) depending on meteorological conditions, traffic effects and horizontal curve parameters for a year on a monthly basis. Within this context, two different roads were investigated and measured with dynamic cone-penetrometer. The total number of the measurement points was 265 for traffic-restricted road (road no: 001-RN1) and 315 for open traffic road (road no: 005-RN2). In the study, three multiple regression models were developed to estimate PBC values on forest road. According to Model1, which was developed to estimate PBC values depending on vehicle traffic and on meteorological factors for alignment section of the RN2, the adjusted R² was found to be 0.635. In Model2 for the curve section of the RN2, the adjusted R² was found to be 0.711. Model3 for RN1 depending on meteorological factors demonstrated that the accuracy of PBC estimation had a high adjusted R², which was 0.952. In conclusion, PBC values can be estimated at high accuracy. Furthermore, traffic load has a strong effect on PBC. On the other hand, temperature has an important negative effect on the variation in PBC on RN1.

Keywords: *forest road, meteorological data, traffic volume, bearing capacity, pavement*

Introduction

Roads provide access for people to work, enjoy, or consider natural ecosystems (Demir, 2007). Forest road ecosystem includes both the paved and unpaved rights of way and adjacent structure, including other infrastructure, ditches, drainage features, and other components that provide the means for vegetation to establish and provide habitat for associated plants and animals (Lugo and Gucinski, 2000). Forest roads have several functions with respect to the management of forestry activities (Acar, 2016; Demir and Hasdemir, 2005). One of these main functions is the transport of timber from its point of felling to the mill. It accounts for a high proportion of the

costs to the industry (Dawson, 2001). For that reason, different heavy vehicles operate on forest roads to manage forestry activities and forest operations. On the other hand, therefore, so as to fulfil these functions, pavement is an important element on forest roads (Akay et al., 2018). However, land degradation and pavement deteriorates in time depending on climate factors, traffic load, maintenance works, slope degree, canopy closure and other factors (Haas, 2001; Tighe et al., 2003; Akgul et al., 2017; Akgul and Hasdemir, 2018; Gokbulak et al., 2018; Sheikh et al., 2017; Yurtseven et al., 2019). Also, pavement deterioration occurs depending on pavement surface compaction rate. Pavement deterioration is the most important factor for traffic safety and safe drive of vehicles. Pavement performance changes depending on deterioration (Akgul et al., 2016).

Pavement deterioration can be slowed down or stopped with proper maintenance. For this reason, it is essential to evaluate the structural condition of a pavement, for example its bearing capacity (Domitrovic and Rukavina, 2013). On the other hand, Kiss et al. (2016) emphasized that in order to prevent significant road deterioration, high bearing capacity is required for the roads which are intensively used by the vehicles. The bearing capacity of a pavement system is defined as the number of wheels passages that it can support before it reaches structural distress (O'mahony et al., 2000). Direct measurement of the bearing capacity is not possible. Instead, the deflection caused by a known load can be measured, and then the bearing capacity can be calculated (Primusz et al., 2015). Most of the devices used to measure the bearing capacity of forest roads express the measurement results with regard to elastic modulus (Kaakkurivaara et al., 2015)

The bearing capacity of a pavement mainly depends on its structure (Trzcinski and Kaczmarzyk, 2006). Besides, the diversity of traffic load and extreme meteorological conditions affect pavement structure, for example its bearing capacity (Bocz, 2009). The stresses caused by traffic load affect each pavement layer differently. For example, it causes deformations and structural changes in the pavement. Climate conditions significantly affect pavement stiffness and bearing capacity (Szentpeteri, 2013). In spring, the bearing capacity of pavement decreases because of the increase in the amount of moisture on subgrade (Charlier et al., 2009; Vestin et al., 2018). The bearing capacity of pavement is easily determined by a Dynamic Cone Penetrometer (DCP). On the other hand, DCP is a simple testing device for measuring in situ compaction, density, strength or stiffness (Wu and Sargand, 2007; Puppala, 2008).

The objective of the study was to monitor the changes in forest road pavement bearing capacity (PBC) on two different forest roads depending on meteorological conditions, traffic effects (vehicle passages, traffic load, vehicle tonnage etc.) and road sections (horizontal curve and alignment).

Material and methods

Study area

The research area is located in the northern part of Istanbul University's Education Research and Practice Forest close to Sariyer, Istanbul. The research field is at Thracian side of the Marmara Region between 28° 59' 17" – 29° 32' 25" east longitudes and 41° 09' 15" – 41° 11' 01" north latitudes according to Greenwich. Within the scope of the study, two different forest roads [road no: 001 (RN1) and road no: 005 (RN2)] were selected as study areas (*Fig. 1*).

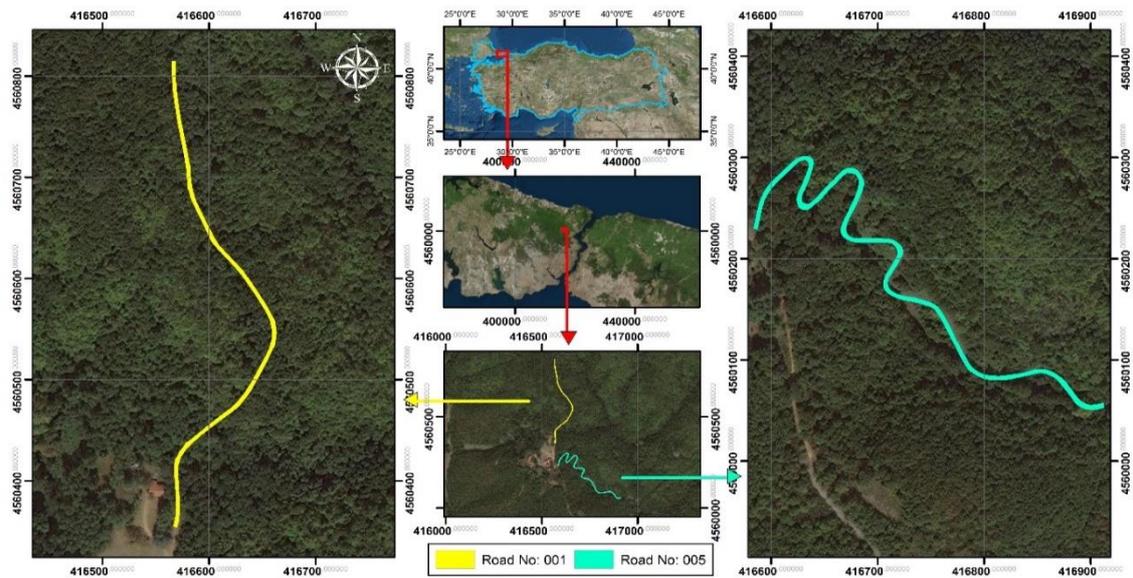


Figure 1. Location map of the study area

The RN1 and RN2 were classified as Normal Type-B forest road with 4-m platform width. The total length of the RN1 was 530 m while the total length of the RN2 was 684 m. The average slope of the RN1 was 8%, while the average slope of the RN2 was 12%. The RN1 was composed of nine horizontal curves while RN2 was composed of eleven horizontal curves. The minimum curve radius was 9.0 m while maximum curve radius was 200 m for RN1, and the minimum curve radius was 6.8 m while maximum curve radius was 50.3 m for RN2 (*Table 1*).

Table 1. Geometrical specifications of roads

	No	Type	Length (m)	Radius (m)	Direction (grad)	Start station (m)	End station (m)	Delta angle (grad)	Chord length (m)	Degree of curvature by arc (grad)
RN1	1	Alignment	8.521		15.064	0+000.00	0+008.52			
	2	Curve	11.285	39.602		0+008.52	0+019.81	18.140	11.246	48.226
	3	Alignment	31.272		396.924	0+019.81	0+051.08			
	4	Curve	38.758	42.441		0+051.08	0+089.84	58.136	37.425	44.999
	5	Alignment	25.245		55.060	0+089.84	0+115.08			
	6	Curve	78.910	200		0+115.08	0+193.99	25.117	78.399	9.549
	7	Alignment	19.991		29.9432	0+193.99	0+213.98			
	8	Curve	24.327	23.971		0+213.98	0+238.31	64.608	23.296	79.675
	9	Alignment	36.185		365.334	0+238.31	0+274.49			
	10	Curve	35.593	200		0+274.49	0+310.09	11.329	35.546	9.549
	11	Alignment	13.724		354.004	0+310.09	0+323.81			
	12	Curve	18.178	64.663		0+323.81	0+341.99	17.896	18.118	29.535
	13	Alignment	8.440		371.901	0+341.99	0+350.43			
	14	Curve	65.77	200		0+350.43	0+416.20	20.936	65.478	9.549
	15	Alignment	0.386		392.837	0+416.20	0+416.59			
	16	Curve	0.515	9.066		0+416.59	0+417.10	3.614	0.515	210.652
	17	Alignment	56.472		389.223	0+417.10	0+473.57			
	18	Curve	28.090	200.000		0+473.57	0+530.00	8.941	28.067	9.549
	19	Alignment	15.711		398.164	0+501.66	0+517.38			

RN2	1	Alignment	21.816		13.081	0+000.00	0+021.82					
	2	Curve	28.317	50.353		0+021.82	0+050.13	35.801	27.945	37.929		
	3	Alignment	30.400		48.882	0+050.13	0+080.53					
	4	Curve	22.514	8.161		0+080.53	0+103.05	175.619	16.024	234.015		
	5	Alignment	30.161		224.502	0+103.05	0+133.21					
	6	Curve	18.797	6.830		0+133.21	0+152.00	175.195	13.402	279.609		
	7	Alignment	40.883		49.306	0+152.00	0+192.89					
	8	Curve	32.824	11.772		0+192.89	0+225.71	177.500	23.178	162.231		
	9	Alignment	38.428		226.807	0+225.71	0+264.14					
	10	Curve	22.304	11.294		0+264.14	0+286.44	125.716	18.852	169.096		
	11	Alignment	30.312		101.090	0+286.44	0+316.75					
	12	Curve	23.562	11.457		0+316.75	0+340.32	130.919	19.624	166.691		
	13	Alignment	22.422		232.010	0+340.32	0+362.74					
	14	Curve	23.553	12.642		0+362.74	0+386.29	118.612	20.292	151.075		
	15	Alignment	26.817		113.398	0+386.29	0+413.11					
	16	Curve	21.074	23.375		0+413.11	0+434.18	57.395	20.368	81.704		
	17	Alignment	49.803		170.793	0+434.18	0+483.99					
	18	Curve	44.610	33.759		0+483.99	0+528.60	84.123	41.434	56.572		
	19	Alignment	28.883		86.670	0+528.60	0+557.48					
	20	Curve	29.577	25.601		0+557.48	0+587.06	73.550	27.96	74.601		
	21	Alignment	20.584		160.221	0+587.06	0+607.64					
	22	Curve	23.604	19.822		0+607.64	0+631.24	75.800	22.234	96.350		
	23	Alignment	53.464		84.412	0+631.24	0+684.71					

Meteorological data (weather conditions)

Weather data was continuously recorded at the adjacent weather station at the Green Roof Research Site located in Istanbul University Faculty of Forestry. Weather data was measured by an automated weather station (DeltaOhm HD2003). Precipitation measurements (hourly basis) were performed using a rain gauge (DeltaOhm HD2003 tipping bucket, measurement accuracy $\pm 1\%$). Also, weather data recorded during study period were compared to long term meteorological data between 1929 and 2017 for the research field which were listed in *Table 2* (General Directory of Meteorology-GDM, 2018).

Table 2. Long term meteorological data from Istanbul (1929-2017) (GDM, 2018)

	Months												Annual
	I	II	III	IV	V	VI	VII	VIII	IX	X	XI	XII	
Mean temperature (°C)	6.0	6.1	7.7	12.0	16.7	21.4	23.8	23.8	20.1	15.7	11.7	8.3	14.4
Maximum temperature (°C)	8.4	9.0	10.9	15.4	20.0	24.6	26.6	26.8	23.7	19.1	14.8	10.8	17.5
Minimum temperature (°C)	3.1	3.1	4.2	7.6	12.1	16.5	19.4	20.1	16.8	12.9	8.9	5.5	10.8
Precipitation (mm)	106.0	77.7	71.4	45.9	34.4	36.0	33.3	39.9	61.7	88.0	100.9	122.2	817.4

Data collection for traffic characterization

Within the scope of the study, camera traps were installed to observe traffic characterization of the RN2. The RN1 was restricted to vehicle passages for one year. The camera traps (Bushnell Trophy Cam, 8MP) were positioned on tree trunks at both the start and end section of the RN2. The camera traps were programmed to take photo at 1 sec interval. The pictures taken by the camera traps were controlled on a monthly basis from September 2015 to September 2016. Vehicle tonnages were calculated in

four groups (automobile, crossover & SUV, minibus, pickup) according to vehicle types.

Measurement of PBC and data collection

In this study, in order to measure PBC, 53 measurement lines (30 lines in curve section; 23 lines in alignment section) at 10-m intervals were determined along the RN1 while 63 measurement lines (29 lines in curve section; 34 lines in alignment section) at 10-m intervals were determined along the RN2 (*Fig. 2*).



Figure 2. Penetrometer measuring points

Five penetrometer measurement points were set on each measurement line with a right angle to the road platform. In total, 265 measurement points (150 points in curve section; 115 point in alignment section) were established for the RN1 and 315 measurement points (145 points in curve section; 170 points in alignment section) were established for the RN2.

The measurement points were fixed with nails (20 cm length). The coordinates of each measurement point were measured by Pentax W800 total station in UTM ED1950 coordinate system (*Fig. 3*).



Figure 3. Establishment of measurement points

PBC was measured using a penetrometer (Eijkelkamp Agrisearch Equipment, the Netherlands). Thirty-degree cones were used with a cone basal surface area of 1 cm^2 (nominal diameter 11.28 mm) (Fig. 4). Data collection was conducted from October 2015 to September 2016 on a monthly basis.

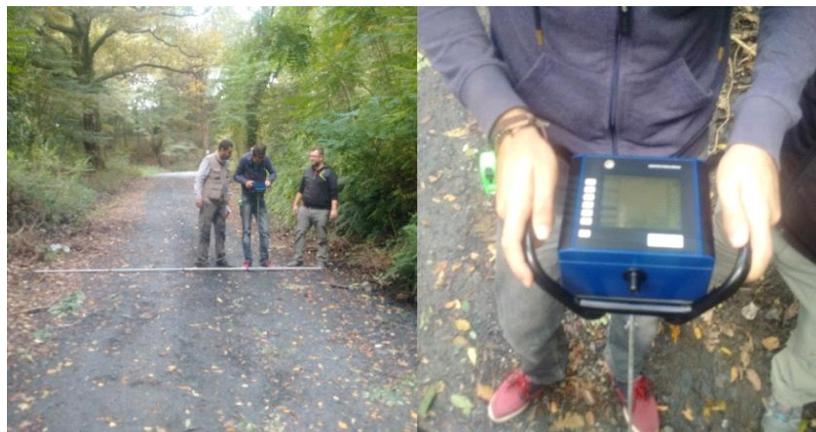


Figure 4. Data collection

Statistical analysis

All statistical analyses were performed using SPSS 23.0 statistical package. In order to estimate the effects of road geometrical properties on PBC, two different multiple regression models were developed for both road alignments and curves on the RN2. Moreover, multiple linear regression model was developed for the in RN1 to estimate the effects of meteorological parameters on PBC. To evaluate the accuracy of the mathematical model by regression analysis, 80% of the total number of the variables ($N = 1668$ for alignment section of RN2; $N = 1371$ for curve section of RN2; $N = 2540$ for RN1) were randomly selected and used as calibration data, while 20% of them ($N = 432$ for alignment section of RN2; $N = 369$ for curve section of RN2; $N = 640$ RN1) were also used as testing data. Furthermore, paired sample *t*-Test and correlation analysis were used to calculate the significance level of the models.

Results and discussion

Results of meteorological data

According to the meteorological data, the minimum average temperature was measured in February as $6.50 \text{ }^{\circ}\text{C}$ while the maximum was measured in August as $24.52 \text{ }^{\circ}\text{C}$. The maximum total precipitation was measured in February as 130.1 mm while the minimum was measured in September as 9.6 mm (Table 3).

Results of traffic characteristics

According to the camera traps, 4598 vehicle passages were observed from September 2015 to September 2016 (Table 4; Fig. 5). The minimum total vehicle passages were in January as with 80 passages while the maximum total vehicle passages on monthly basis was in September 2016, which reached 1604 passages. Moreover, the average speed of vehicles was calculated as 30 km/h for this road.

Table 3. Meteorological data

Year	Months	Mean temperature (°C)	Total precipitation (mm)	Rainfall duration (h)	Precipitation intensity (mm/h)	Mean pressure (Mbar)
2015	November	19.19	143.4	359	0.40	987.46
	October	13.75	75.5	155	0.48	992.48
	December	10.52	56	223	0.25	991.78
2016	January	6.66	87.3	365	0.240	991.48
	February	6.50	130.1	399	0.33	990.25
	March	10.70	63.4	177	0.36	985.77
	April	11.80	35.6	125	0.28	985.20
	May	15.36	40.6	154	0.26	984.20
	June	18.89	50.8	109	0.46	983.75
	July	23.72	45.1	73	0.62	985.03
	August	24.52	34.3	42	0.82	984.47
	September	23.47	9.6	36	0.27	986.18

Table 4. Traffic characteristic of RN2

Month	Year	Automobile (1250 kg)		SUV-Crossover (2000 kg)		Minibus (2000 kg)		Pickup (3300 kg+)		Total passes	Total Tonnage (ton)
		Passes	Tonnage	Passes	Tonnage	Passes	Tonnage	Passes	Tonnage		
		September-October	2015	98	122500	28	56000	42	84000		
October-November	106	132500		20	40000	50	100000	22	72600	198	345100
November-December	172	215000		88	176000	70	140000	78	257400	408	788400
December-January	26	32500		6	12000	20	40000	28	92400	80	176900
January-February	2016	90	112500	52	104000	108	216000	70	231000	320	663500
February-March		190	237500	46	92000	88	176000	54	178200	378	683700
March-April		204	255000	60	120000	56	112000	58	191400	378	678400
April-May		110	137500	62	124000	94	188000	66	217800	332	667300
May-June		144	180000	58	116000	100	200000	54	178200	356	674200
June-July		204	255000	16	32000	12	24000	16	52800	248	363800
July-August		140	175000	48	96000	8	16000	4	13200	200	300200
August-September		172	215000	48	96000	8	16000	32	105600	260	432600
September-November		852	1065000	116	232000	408	816000	228	752400	1604	2865400
Mean		193	241154	50	99692	82	163692	58	191908	383	696446

Results of PBC on monthly basis

PBC values measured monthly were calculated as mean values and are listed in Table 5. It was divided into two main columns as curve section and alignment section for monitoring PBC differences between the RN1 and RN2. The table demonstrates that the measured PBC values of the RN1 were lower than those of the RN2. Furthermore, the measurement results showed that PBC values of the curve sections were relatively lower than those of the alignment sections on the RN2. A comparison of PBC values between the curve section of the RN1 and that of the RN2 revealed that PBC values of the RN2 was almost two times higher than those of the RN1 (Fig. 6).



Figure 5. Sample pictures which were taken from camera traps

Table 5. Mean PBC values on curves and alignment for RN1 and RN2

Month	RN2						RN1					
	Curve section			Alignment section			Curve section			Alignment section		
	Side Zone (MPa)	Center Zone (MPa)	Rut Zone (MPa)	Side Zone (MPa)	Center Zone (MPa)	Rut Zone (MPa)	Side Zone (MPa)	Center Zone (MPa)	Rut Zone (MPa)	Side Zone (MPa)	Center Zone (MPa)	Rut Zone (MPa)
October 2015	1.985	2.000	1.995	2.036	2.037	2.045	0.582	0.594	0.589	0.570	0.563	0.569
November 2015	2.207	2.234	2.233	2.204	2.202	2.213	0.976	0.991	0.984	0.973	0.985	0.988
December 2015	2.080	2.094	2.097	2.321	2.335	2.340	0.860	0.842	0.867	0.872	0.894	0.887
January 2016	1.922	1.950	1.949	2.129	2.134	2.145	0.681	0.691	0.688	0.591	0.600	0.600
February 2016	1.894	1.919	1.919	2.179	2.190	2.197	0.871	0.852	0.864	0.722	0.730	0.722
March 2016	1.811	1.823	1.831	2.157	2.164	2.172	1.049	1.055	1.048	0.987	1.001	0.988
April 2016	1.632	1.650	1.653	1.886	1.905	1.899	0.785	0.779	0.791	0.557	0.570	0.562
May 2016	1.640	1.667	1.667	1.873	1.883	1.887	0.686	0.694	0.688	0.688	0.670	0.695
June 2016	1.915	1.943	1.939	2.030	2.038	2.045	0.664	0.664	0.671	0.868	0.875	0.876
July 2016	1.892	1.921	1.920	1.969	1.980	1.985	0.740	0.745	0.740	0.663	0.678	0.672
August 2016	1.889	1.918	1.917	1.940	1.945	1.959	0.738	0.729	0.735	0.634	0.644	0.639
September 2016	1.970	2.000	1.999	2.073	2.076	2.089	0.588	0.582	0.588	0.494	0.500	0.492
Mean	1.903	1.927	1.927	2.066	2.074	2.081	0.768	0.768	0.771	0.718	0.726	0.724

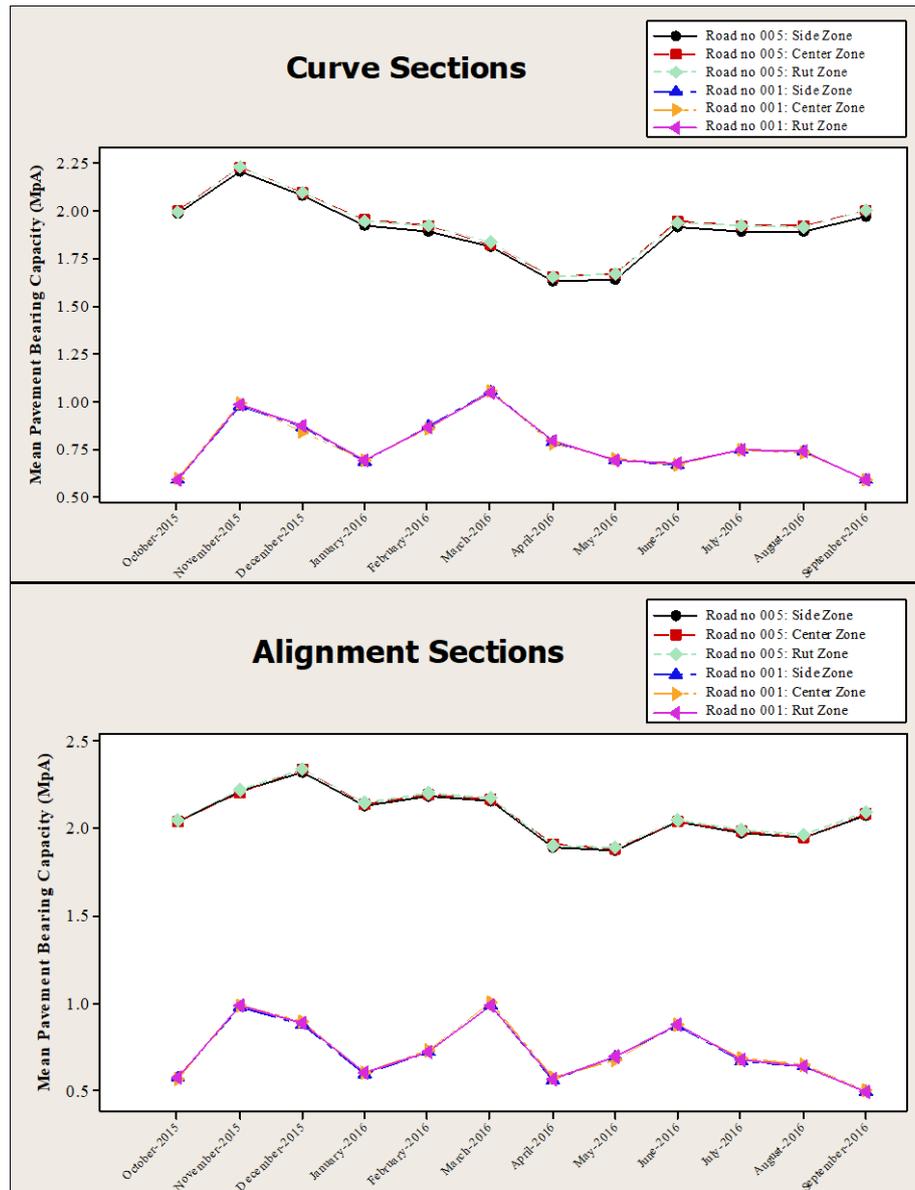


Figure 6. Mean bearing capacity on curve sections and alignment sections of RN1 and RN2

Besides, PBC values of the alignment section on the RN2 were found as three times higher than those on the RN1 (Fig. 7). PBC values were also observed to be lower in spring than in other seasons.

Results of statistical model

The descriptive statistics of the calibration data used in multiple regression model for the curve section ($N = 1371$) of the RN2 are listed in Table 6, while they are presented in Table 7 for the alignment section ($N = 1668$) of the RN2. The descriptive statistics of the calibration data ($N = 2540$) for the RN1 are listed in Table 8.

In order to estimate the effects of road geometrical properties on PBC, two different multiple regression models were developed for both road alignments and curves on the RN2. Moreover, multiple linear regression model was developed to estimate the effects

of meteorological parameters on PBC on the RN1. In all regression models, PBC was considered as the dependent variable. In the regression model for the alignment section of the RN2; road zone (Z), total precipitation (Tp), tonnage (Tn), passages (Ps) were considered as independent variables, in the regression model for the curve section of the RN2; road zone (Z), curve radius (CuR), total precipitation (Tp), precipitation intensity (Pi), passages (Ps), tonnage (Tn), curve length (CuL), while in regression model for RN1 road zone (Z), total precipitation (Tp), temperature (Tm), pressure (Prs) were considered as independent variables.

At the stage of testing the statistical relationship between the variables for the curve section of the RN2, the results of Pearson's correlation coefficients and their significance levels ($p < 0.01$) and correlation analysis are shown in *Table 9*. It was found that there was a strong positive relationship between PBC and $\ln(Z)$, (Z).

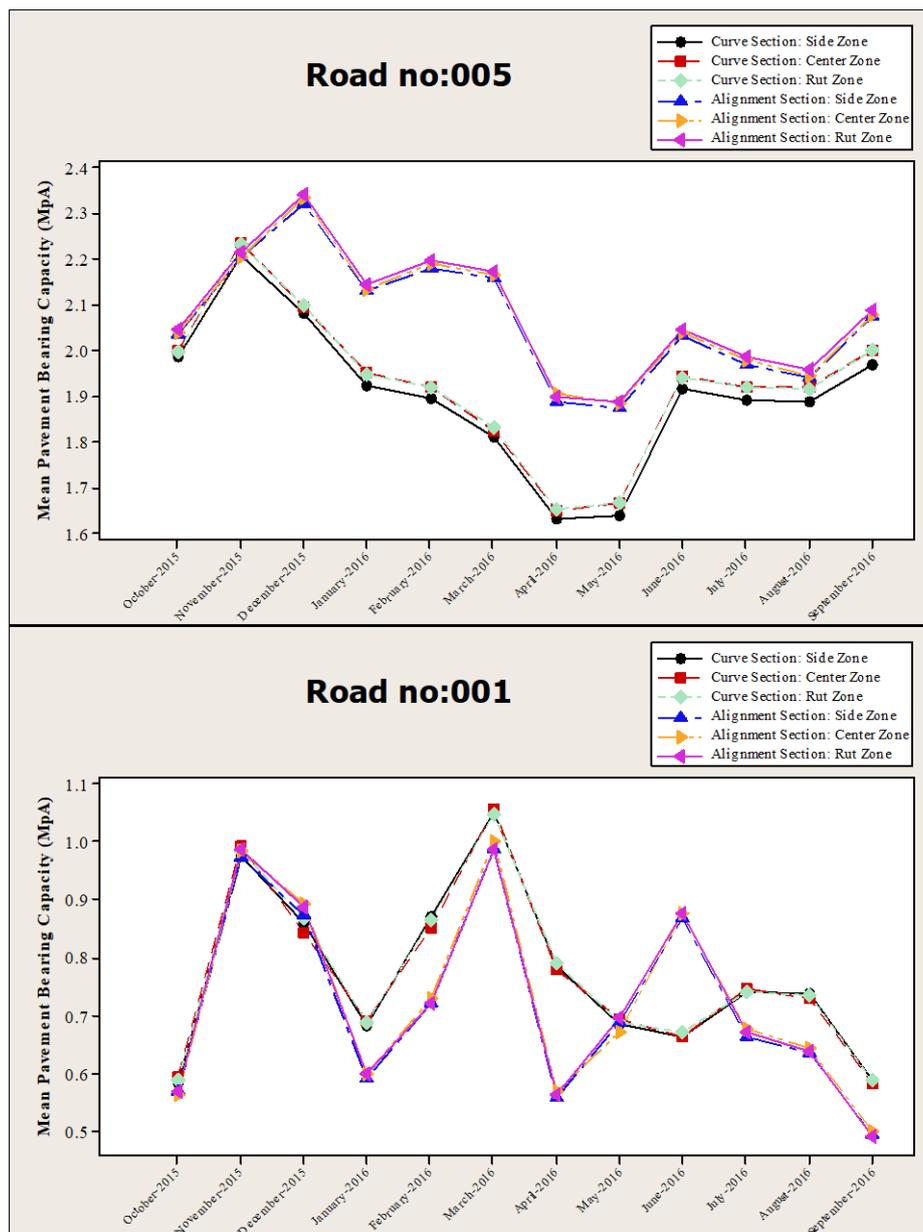


Figure 7. Mean bearing capacity on RN1 and RN2

Table 6. Descriptive statistics for curve section of RN2

RN2 Curve section	N	Minimum	Maximum	Mean		Std. deviation	Variance
	Statistic	Statistic	Statistic	Statistic	Std. Error	Statistic	Statistic
ln(Z) (road zone)	1371	0.693	1.386	0.994	0.006	0.267	0.071
Z (road zone)	1371	2.000	4.000	2.800	0.018	0.749	0.560
CuR (m)	1371	6.830	50.353	21.270	0.312	13.003	169.078
Tp (mm)	1371	9.600	143.400	64.308	0.906	37.803	1429.067
Pi (mm/h)	1371	0.239	0.817	0.398	0.004	0.168	0.028
Ps (passes)	1371	80.000	1604.000	383.170	9.079	378.728	143434.739
Tn (kg)	1371	300.200	2865.400	784.392	16.003	667.537	445605.726
CuL (m)	1371	18.797	44.610	27.882	0.185	1.220	1.488
PBC (MPa)	1371	0.010	3.900	1.918	0.029	7.721	59.612

Table 7. Descriptive statistics for alignment section RN2

RN2 Alignment section	N	Minimum	Maximum	Mean		Std. deviation	Variance
	Statistic	Statistic	Statistic	Statistic	Std. Error	Statistic	Statistic
ln(Z) (road zone)	1668	0.693	1.386	0.992	0.007	0.268	0.072
Z (road zone)	1668	2.000	4.000	2.800	0.018	0.752	0.566
Tp (mm)	1668	9.600	143.400	64.507	0.928	37.910	1437.178
Tn (kg)	1668	300.200	2865.400	781.825	16.300	665.697	443152.172
Ps (passes)	1668	80.000	1604.000	382.140	9.246	377.619	142596.100
PBC (MPa)	1668	0.020	3.996	2.080	0.030	1.244	1.547

Table 8. Descriptive statistics for RN1

RN1 All road sections	N	Minimum	Maximum	Mean		Std. deviation	Variance
	Statistic	Statistic	Statistic	Statistic	Std. Error	Statistic	Statistic
ln(Z) (road zone)	2540	0.693	1.386	0.995	0.005	0.268	0.072
ln(Tp) (mm)	2540	2.262	4.966	3.968	0.014	0.682	0.466
ln(Tm) (°C)	2540	-1.431	3.200	1.064	0.037	1.852	3.409
ln(Prs) (Mbar)	2540	4.238	6.900	5.785	0.025	1.266	1.602
ln(PBC) (MPa)	2540	-4.605	6.900	2.513	0.078	3.945	15.564

Table 9. Correlations between variables for curve section of RN2

	ln(Z)	Z	CuR	Tp	Pi	Ps	Tn	CuL	PBC
ln(Z)	1								
Z	0.994	1							
CuR	0.009	0.010	1						
Tp	-0.006	-0.007	-0.001	1					
Pi	-0.005	-0.006	-0.005	-0.143	1				
Ps	0.014	0.015	0.018	-0.479	-0.289	1			
Tn	0.015	0.016	0.013	-0.370	-0.329	0.919	1		
CuL	0.004	0.006	0.495	0.017	-0.002	0.004	-0.005	1	
PBC	0.758*	0.718*	0.024	0.031	0.029	0.007	0.035	-0.090	1

*Correlation is significant at the 0.01 level

Also, according to Pearson's correlation coefficients and their significance levels ($p < 0.01$), a strong positive correlation was found between PBC and $\ln(Z)$, (Z) (Table 10).

Table 10. Correlations between variables for alignment section of RN2

	$\ln(Z)$	Z	Tp	Tn	Ps	PBC
$\ln(Z)$	1					
Z	0.995	1				
Tp	-0.005	-0.005	1			
Tn	0.010	0.011	-0.367	1		
Ps	0.013	0.013	-0.477	0.917	1	
PBC	0.713*	0.673*	0.036	0.042	0.019	1

*Correlation is significant at the 0.01 level

Considering the correlation analysis results evaluated for the RN1, their significance levels ($p < 0.01$), there was a weak correlation between PBC and Z unlike the RN2 (Table 11). However, a strong negative correlation was found between PBC and Tm , Prs .

Table 11. Correlations between variables for RN1

	$\ln(Z)$	$\ln(Tp)$	$\ln(Tm)$	$\ln(Prs)$	$\ln(PBC)$
$\ln(Z)$	1				
$\ln(Tp)$	-0.011	1			
$\ln(Tm)$	0.000	-0.072	1		
$\ln(Prs)$	-0.002	0.012	0.973	1	
$\ln(PBC)$	0.069*	0.003	-0.951*	-0.973*	1

*Correlation is significant at the 0.01 level

F test and adjusted R^2 statistics of the multiple linear regression models indicated that all three models were effective predictors of PBC. According to model 1, which was developed to estimate PBC values depending on vehicle traffic effects and meteorological factors for the alignment section of the RN2, adjusted R^2 was found as 0.635. Moreover, in Model 2 developed to estimate PBC values depending on vehicle traffic effects, curve parameters and meteorological factors for the curve section of the RN2, adjusted R^2 was found as 0.711. Model 3, which was developed to estimate PBC values for all sections of the RN1 depending on meteorological factors, adjusted R^2 was found as 0.952 (Tables 12 and 13).

Table 12. Statistical summary of regression model RN1 and RN2

Road No	Road section	N	Adjusted R^2	Std. error of the estimate	F	Sig.
RN2	Alignment	1668	0.635	0.750	582.219	0.000
RN2	Curve	1371	0.711	0.664	421.854	0.000
RN1	All road	2540	0.952	0.866	12519.627	0.000

Table 13. Summary of regression model coefficient RN1 and RN2

Model no	Road no	Road section	Model	Unstandardized coefficients		Regression model
				B	Sig.	
Model 1	RN2	Alignment	Constant	-1.214	0.000	$Y = -1.214 + 18.684 \ln(Z) - 5.519Z + 0.002Tp + 0.0002Tn + 0.0003Ps$
			ln(Z)	18.684	0.000	
			Z	-5.519	0.000	
			Tp	0.002	0.007	
			Tn	0.0002	0.000	
			Ps	0.0003	0.019	
Model 2	RN2	Curve	Constant	-1.387	0.000	$Y = -1.387 + 18.336 \ln(Z) - 5.326Z + 0.021CuL + 0.008CuR + 0.002Tp + 0.0003Tn + 0.427Pi - 0.0004Ps$
			ln(Z)	18.336	0.000	
			Z	-5.326	0.000	
			CuL	-0.021	0.000	
			CuR	0.008	0.000	
			Tp	0.002	0.005	
			Tn	0.0003	0.000	
			Pi	0.427	0.000	
Ps	-0.0004	0.002				
Model 3	RN1	All Road	Constant	17.867	0.000	$Y = e^{17.867 + 1.001 \ln(Z) - 2.849 \ln(Prs) - 0.130 \ln(Tm) + 0.065(Tp)}$
			ln(Z)	1.001	0.000	
			ln(Prs)	-2.849	0.000	
			ln(Tm)	-0.130	0.003	
			ln(Tp)	0.065	0.016	

As it is seen in *Table 12*, according to results of RN1's adjusted R^2 it was possible to closely estimate PBC without traffic effects on forest road.

Also, in the study, developed regression models validated with test data (observation data). Scatter plot for model 1 had a linear correlation with $R^2 = 0.57$ between the observed and predicted PBC ($N = 432$), scatter plot model 2 had a linear correlation with $R^2 = 0.59$ between the observed and predicted PBC ($N = 369$) while scatter plot for model 3 had a linear correlation with $R^2 = 0.96$ between the observed and predicted PBC ($N = 640$) (*Figs. 8, 9, and 10*).

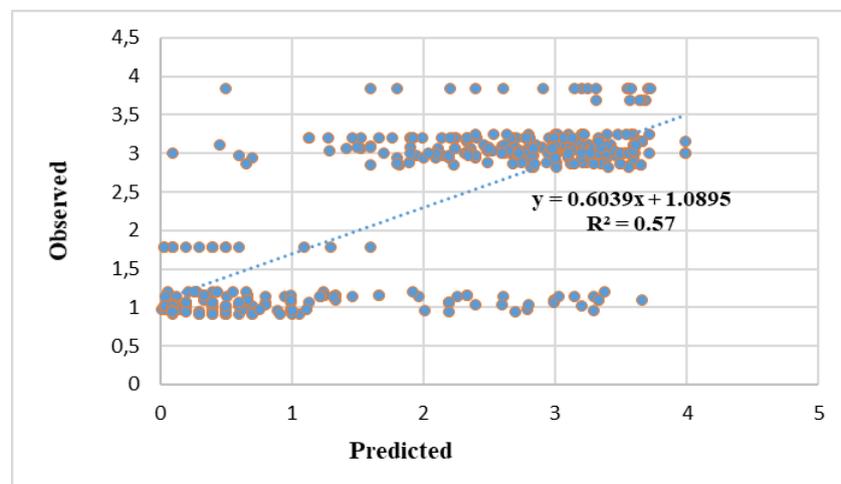


Figure 8. Validation of predicted and observed PBC values for model 1

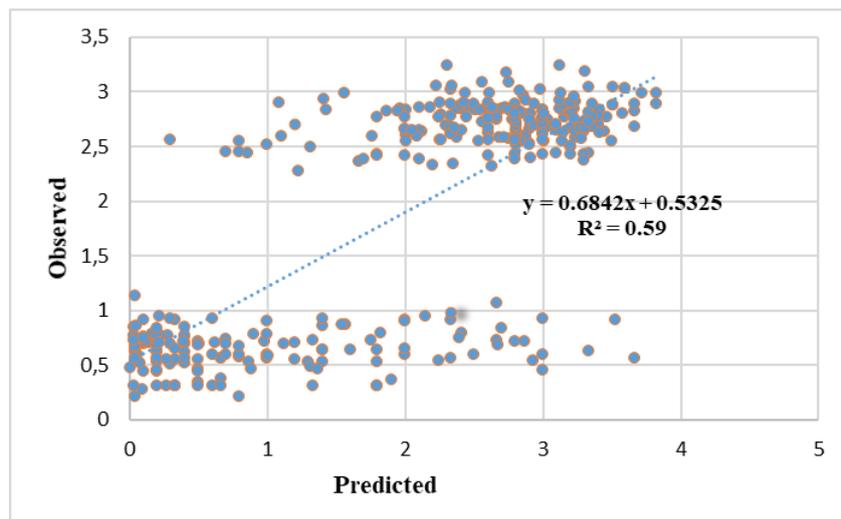


Figure 9. Validation of predicted and observed PBC values for model 2

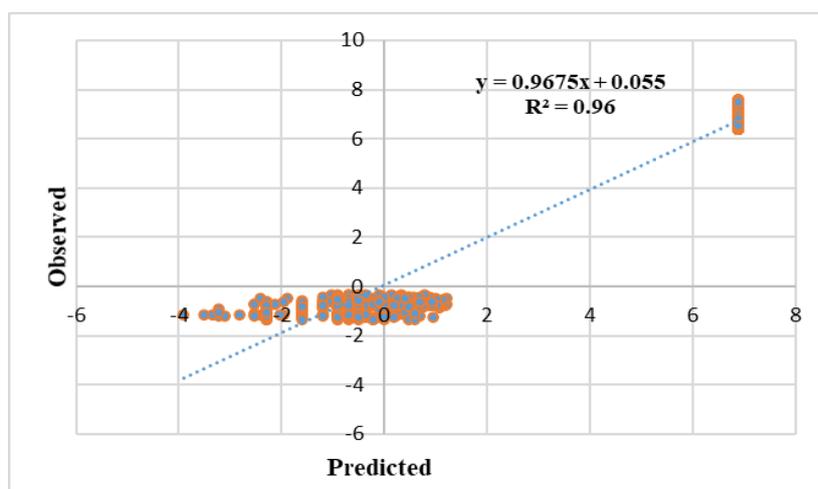


Figure 10. Validation of predicted and observed PBC values for model 3

Within the scope of the study, two different forest roads were investigated for monitoring the changes in PBC in one year. One of these roads was restricted to vehicle passages during the study. Hence, on this road (RN1); the effects of meteorological factors on PBC were investigated. Furthermore, on the other road (RN2), the changes in PBC were monitored depending on road sections, traffic effects and meteorological effects.

A comparison of the mean PBC between two roads, it was observed that PBC values on the RN2 were almost two or three times higher in the RN1. The most likely reason might be traffic load on the RN2. Similarly, Săceanu (2012) found a strong correlation between PBC and vehicle passages and tonnage. Our results showed that traffic load (vehicle passages and tonnage) was an important factor. In addition, during the measurement period on the RN2 which is open to traffic, there is a general decrease in the overall PBC values caused by the traffic load. In this regard, Salour (2015) stated that the combination of environmental factors and intensive vehicle traffic increased the

deterioration of the road structure. The PBC values of the curve sections that we found were relatively lower than those of the alignment sections on the RN2. The possible reasons for that difference might include the changes (braking, accelerating) in the speed of the vehicles in the curve and alignment regions and lateral shifts of vehicles. Moreover, Das et al. (2016) reported that vehicle of placement in curve was different when compared to alignment, and the radius of curve also affected the lateral position of the vehicle.

PBC values were observed to be lower in spring than in other season. In order to explain the reason for this, Charlier et al. (2009) emphasized that the PBC decreases in spring because of the increase in the amount of moisture on subgrade. On the other hand, in connection with this, Yoshida et al. (2016) mentioned the need to have adequate drainage facilities to maintain the sustainability of the bearing capacity. Adlinge and Gupta (2013) also stated that moisture significantly reduced the strength of subgrade. Demir et al. (2012) and Erdem et al. (2018) stated that there is a parallel relationship to between monthly precipitation and sediment production. Monthly sediment production from unpaved forest road was significantly higher than that of paved forest road and undisturbed area. It clearly shows that a stabilizing cover on a forest road led to less sediment production and more soil protection (Demir et al., 2012; Erdem, 2018). The results of previous studies are similar to ours (Kaakkurivaara et al., 2015; Salour, 2015; Grajewski, 2016).

According to the correlation of the results, PBC values had a high negative correlation with Temperature (T_m) on RN1. Similar to these results, Pan et al. (2015) and Motiejūnas et al. (2010) reported that the increase in temperature on asphalt roads led to a decrease in the PBC. On the RN2, there was a strong relationship between PBC and road zone (Z) values both in the curves and alignments. On the RN1, however, there was a weak relationship between PBC and zone (Z). This might be probably because there was no traffic load on the RN1 while there was traffic load on the RN2. According to the correlation results, PBC values had a high correlation with road zone. We have the impression that it was possibly due to the wheel track.

Conclusions

Road surface stability is an important factor on sustainability of forest operations in all seasons. In this context, the PBC value is an indication of the continuity of forest operations. Also, these values and models can be useful to managers for decision making stage in forestry activities.

So, traffic load, especially vehicle tonnage had a strong effect on PBC. On the other hand, temperature had a significant effect on the variation in PBC on the RN1. However, three multiple regression models were developed to estimate PBC values. According to the regression models, PBC values could be estimated at high accuracy. These models will also provide useful results to managers for decision-making in the future. Moreover, fixed measurement points constituted in study can be used to generate pavement compaction maps, PBC changes map etc. for future studies.

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